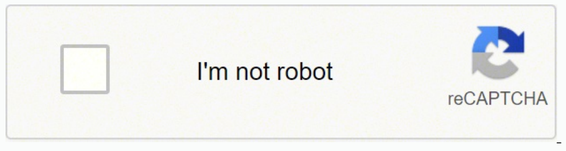

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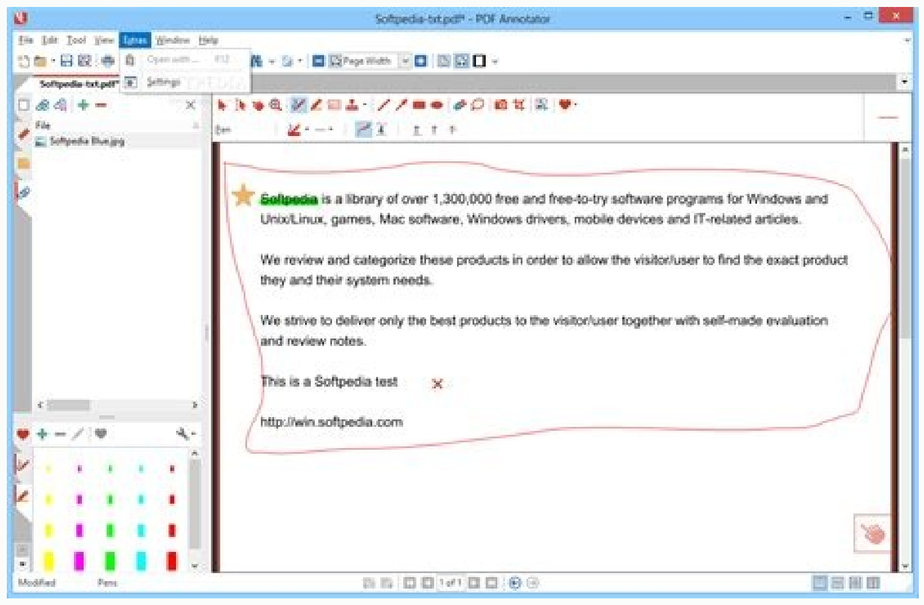

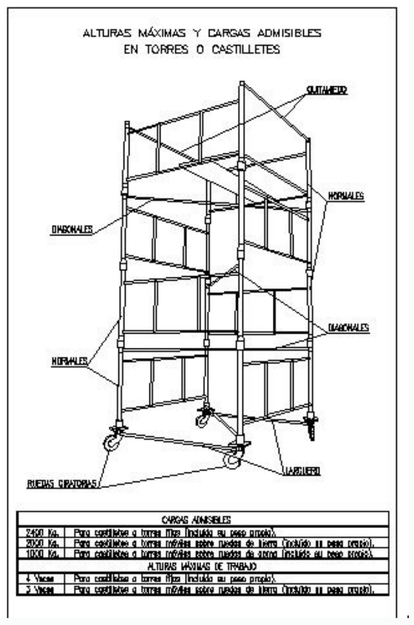

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   5. Depth/Pressure Gauge (Digital)
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ii. Underwater

iii. J.W. Luscher Mfg.
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   2. Boat Towable Metal Detector
   3. Boat Towable Metal Detector
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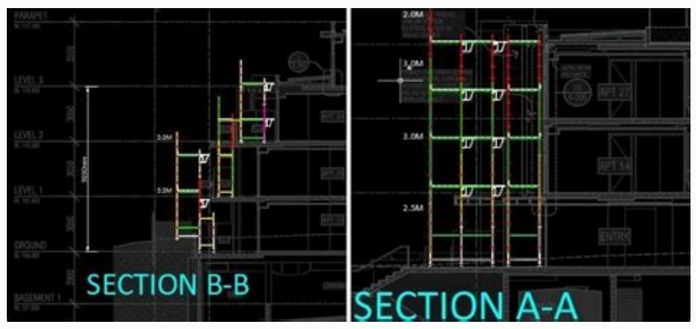

Types of scaffolding and their uses. Types of scaffolding with diagram. Types of scaffolding pdf download. Types of scaffold coupler.

This scaffolding structure consist 3 platform at 10m, 14m and 18m. Plywood Timber batten * Live load Construction load Thus, total service load is 2.0 kN/m = 25 kN/m³ × 0.15m × 0.1m = = 10 kN/m³ × 0.012m × 0.1m = = 10 kN/m³ × 0.10m × 0.050m = 0.375 kN/m 0.012 kN/m 0.05 kN/m = 2.5 kN/m² × 0.3m 0.75 kN/m 1.187 kN/m = 5.2 Bending Stress w = 2.0 kN/m L = 1.8 m * D/B = 100 / 50 = 2.0 < 5 (table 3.9) Side stability ----------o.k * Mmax = wL² / 8 = (2.0 × 1.8²) / 8 = 0.81 kNm * Section modulus, Z = (BD²) / 6 = (50 × 100²) /6 = 0.833 × 10^5 mm³ * Actual bending stress, fs = M/Z = (0.81 × 10^6) / (0.833 × 10^5) = 9.72 N/mm² * Allowable stress, fp = fg × kl × kxb × k4 × k5 where fg = 12.41 N/mm² k1 =1 (table 3.6) kxb = 1.1 (spacing < 610mm Clause 3.9c) k4 =1 k5 =1 Thus, fp = 12.41 × 1 × 1.1 × 1 × 1 = 13.65 N/mm² > fs ----------o.k 5.3 Shear stress * Shear force, V * Shear stress, qs = wL/2 = (2.0 kN/m × 1.8m) /2 = 0.54 N/mm² = 1.5V / A = (1.5 × 1.8 × 10³) / (100 × 50) = 0.54 N/mm² * Allowable shear stress, qp = 1.29 N/mm² Since qs < qp ---------- o.k 6.0 Formwork for Concrete Floor Beam 6.1 Design of timber batten to support wet concrete floor beam (say 1000mm × 800mm) (75mm × 75mm @ 600mm c/c spacing) 6.1.1 Loading * Dead load R.C. slab 12mm thk. 2020 5 Project : PARADIP IOCL SECTION: Calculation SHEET 3OF 12 Scaffolding Design STRUCTURAL DESDRIPTION This scaffolding structure is 1.7m X 3.0m plan in size and height 18.0m. NO. 2020 Project : PARADIP IOCL SECTION: Calculation SHEET 1OF 12 Scaffolding Design TABLE OF CONTENTS 1 INTRODUCTION .....02 2 SCOPE .....02 3 REFERENCE AND STANDARDS .....02 4 DESIGN BASIS .....02 5 STRUCTURAL DESCRIPTION .....03 6 STAAD MODELING .....03 6.1 STAAD MODEL .....03 6.2 STAADMODEL .....04 6.3 STAAD MODEL WITH MEMBER PROPERTIES.....05 7 LOAD CALCULATION .....06 7.1 DEAD LOAD .....06 7.2 LIVE LOAD .....06 7.3 WIND LOAD .....06 7.4 SEISMIC LOAD .....06 7.5 PRIMARY LOADS AND COMBINATION.....06 8 STAAD INPUT .....07 9 UTILITY RATIO TABLE .....10 TUAMAN ENGINEERING LIMITED DOC:001/04.06. 2020 41 42 43 44 45 46 47 48 49 50 51 52 53 54 55 56 57 58 59 60 61 62 63 64 65 66 67 68 69 70 71 72 73 74 75 76 77 78 79 80 81 82 83 84 SECTION: Calculation SHEET 11OF 12 Project : PARADIP IOCL Scaffolding Design PIP424M PIP424H PIP424M PIP424M PIP424M

0.355 0.352 0.352 0.355 0.352 Sec. TUAMAN ENGINEERING LIMITED DOC:001/04.06.20 20 Project : PARADIP IOCL SECTION :Cover sheet SHEET I OF I Scaffolding Design TUAMAN ENGINEERING LIMITED Design of Calculation for Scaffolding Document No. : 001/04.06.2020. The platform is made using gratings. 8.2.1.2 Sec. R.C. slab 12mm thk. Hence minimum wind load is considered on scaffolding members. 1.0 Design Data 1.1 Dead Load i) Density of concrete ii) Density of timber iii) Scaffolding vertical frame iv) Steps for scaffolding = = = = 1.2 Live Load i) Construction load = 12.5 kN/m² 24 kN/m³ 10 kN/m³ 0.25 kN/pc 0.15 kN/pc 1.3 Other relevant data i) No wind load is taken in account ii) Timber Grade B Standard to be used (moisture content < 19%) 1.4 Scaffolding Scaffolding dimension = 1220 (W) × 1830mm (L) × 1700mm (H) 2.0 Design For External Perimeter Scaffolding 2.1 Scaffolding frames 8 Storey building → Maximum flight of frame (≤ 28) Dead Load Vertical Frame Steps Cross Brace Joint Pin Jack Base Tie Back Safety Net : : : : : : : Live Load 2 Workers Construction debris & material Total Load 25 kg × 28 15 kg × 28 5 kg × 28 × 2 0.5 kg × 27 × 2 6.8 kg × 2 2.2 kg × 10 (36 × 1.8) × 2 kg Total = = = = = = = 700 420 280 27 13.6 22 130 1592.6 : 75 kg × 2 : = = 150 100 250 1250 : = = kg kg kg kg kg kg kg kg kg kg (assume) kg × 5 (assume 5 working level) × 1.6 kg 4229.6 kg 42.3 kN < 50 kN → o.k. (allowable load per frame = 5 ton) → 28 frame of Huatraco HT101 would be sufficient 3.0 Flatform For Scaffolding Up To 46.2m High 3.1 Loading Dead Load Equivalent load = 15.93 kN / (1.7m × 1.22 m) = 7.14 kN/m² Live Load Construction load 3.2 Scaffolding Platform MS RIIS Beam @ 1.8m c/c Dead Load UDL on MS RHS beam Live Load UDL on MS RHS beam = 2.5 kN/m² = 9.4 × 1.8 = 16.92 kN/m = 12.5 × 1.8 = 22.5 kN/m 4.0 Scaffolding Platform Connection 4.1 Welding Using 6mm fillet weld, capacity = 0.7 × 6 × (215 × 10⁻³) = 0.9 kN/mm Critical shear at catch platform truss = 37.26 kN Therefore, welding length required = 37.26 / 1.8 = 20.7 mm 4.2 Bolting Bolt Group Design Design Code : BS5950 - 1990 Analysis : Non Linear Bolt Grade : 4.8 Shear Planes : Single Shear The applied ULS force is : 100 kN The capacity of the bolt groupn is : 141.12 kN .....O.K. The configuration requires 20 mm bolts for a capacity of 141.12 kN Bolt Shear Design is Safe Using HILTI HVU 20mm dia. R0 ISSUE INITIALS PPD BY SIGN INITIALS SIGN. 6 6.1 STAAD MODELING STAAD MODEL TUAMAN ENGINEERING LIMITED DOC:001/04.06. This scaffolding structure is made using pipe member. 9.3.2.2 Sec. 2020 Project : PARADIP IOCL SECTION: Calculation SHEET 9OF 12 Scaffolding Design 1 1.0 2 1.0 5 1.0 LOAD COMB 108 DL+IL+WL(-Z) 1 1.0 2 1.0 6 1.0 ********** PERFORM ANALYSIS PRINT ALL PARAMETER 1 CODE IS800 LSD FU 420000 ALL FYLD 250000 ALL KY 0.9 MEMB 1 TO 6 13 TO 18 25 TO 30 37 TO 42 49 TO 54 61 TO 66 73 TO 78 85 86 TO 90 97 TO 102 109 TO 114 KZ 0.9 MEMB 1 TO 6 13 TO 18 25 TO 30 37 TO 42 49 TO 54 61 TO 66 73 TO 78 85 86 TO 90 97 TO 102 109 TO 114 MAIN 250 ALL CHECK CODE ALL PARAMETER 2 CODE IS800 LSD STEEL MEMBER TAKE OFF ALL FINISH TUAMAN ENGINEERING LIMITED DOC:001/04.06. 2020 6.3 Project : PARADIP IOCL Scaffolding Design STAAD MODEL WITH MEMBER PROPERTIES SECTION: Calculation SHEET 5OF 12 TUAMAN ENGINEERING LIMITED DOC:001/04.06. 2020 7 Project : PARADIP IOCL SECTION: Calculation SHEET 6OF 12 Scaffolding Design LOAD CALCULATION The tank ring foundations are subjected to following loads Dead load Live Load Wind Load Earth quake load 7.1 DEAD LOAD 7.2 Self weight of Structure Selfweight of Gratings 2 = 0.4kN/m LIVE LOAD Load on Platform = 400 kg = (400 x 9.81)/(1000 x 1.7 x 3) = 0.78 kN/m2 7.3 WIND LOAD This scaffolding structure is constructed temporarily for labour access to perform various work. 2020 Project : PARADIP IOCL SECTION: Calculation SHEET 8OF 12 Scaffolding Design 120 TABLE ST PIP424M 3 6 7 9 10 12 15 18 19 21 22 24 27 30 39 42 TABLE ST PIP424H CONSTANTS MATERIAL STEEL ALL SUPPORTS 61 TO 66 PINNED MEMBER RELEASE 1 TO 6 13 TO 18 25 TO 30 37 TO 42 49 TO 54 61 TO 66 73 TO 78 85 TO 90 97 TO 102 109 TO 114 START KMY 10 KMZ 10 1 TO 6 13 TO 18 25 TO 30 37 TO 42 49 TO 54 61 TO 66 73 TO 78 85 TO 90 97 TO 102 109 TO 114 END KMY 10 KMZ 10 DEFINE WIND LOAD TYPE 1 WIND 1 INT 0.5 HEIG 20 EXP 1 JOINT 1 TO 66 LOAD 1 LOADTYPE None TITLE DL SELFWEIGHT Y -1 FLOOR LOAD YRANGE 9 11 FLOAD 0.4 GY YRANGE 13 15 FLOAD 0.4 GY YRANGE 17 19 FLOAD 0.4 GY LOAD 2 LOADTYPE Live TITLE IL FLOOR LOAD YRANGE 9 11 FLOAD 0.77 GY YRANGE 17 19 FLOAD 0.77 GY LOAD 3 LOADTYPE Wind TITLE WL(X) WIND LOAD X 1 TYPE 1 OPEN LOAD 4 LOADTYPE Wind TITLE WL(-X) WIND LOAD X -1 TYPE 1 OPEN LOAD 5 LOADTYPE Wind TITLE WL(Z) WIND LOAD Z 1 TYPE 1 OPEN LOAD 6 LOADTYPE Wind TITLE WL(-Z) WIND LOAD Z -1 TYPE 1 OPEN ********** LOAD COMB 100 DL+IL 1 1.0 2 1.0 LOAD COMB 101 DL+WL(X) 1 1.0 3 1.0 LOAD COMB 102 DL+WL(-X) 1 1.0 4 1.0 LOAD COMB 103 DL+WL(Z) 1 1.0 5 1.0 LOAD COMB 104 DL+WL(-Z) 1 1.0 6 1.0 LOAD COMB 105 DL+IL+WL(X) 1 1.0 2 1.0 3 1.0 LOAD COMB 106 DL+IL+WL(-X) 1 1.0 2 1.0 4 1.0 LOAD COMB 107 DL+IL+WL(Z) TUAMAN ENGINEERING LIMITED DOC:001/04.06.

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